

STRUCTURAL CALCULATION & ANALYSIS EG4® BRIGHTMOUNT ADJUSTABLE GROUND MOUNT SYSTEM

Design Standard: ASCE 7 Installation Site Information

Design Condition and Technical Parameters:

Solar Panel Size	230 x 110 x 3.5cm (90.6 x 43.3 x 1.4in.)
Solar Panel Power	550W
Solar Panel Weight	30 kg (66.1 lbs.)
Basic Wind Speed	47m/s
PV Area	2.53m ²
Span	1.3m

Calculations

Category I = 1.0 ----- Importance Factor, refer to ASCE 7-16, Table 1.5-1

Velocity Pressure due to Wind, q_z :

$q_z = 0.613 K_z K_{zt} K_d K_e V^2$ ----- Velocity Pressure due to Wind, refer to Table 27.3-1, ASCE 7-16

$K_z = 1.0$ ----- Velocity Pressure Exposure Coef. (for structures < 4.57m high, Exposure C)

$K_{zt} = 1.0$ ----- Topographic Factor, refer to Figure 26.8-1, ASCE 7-16

$K_d = 0.85$ ----- Directionality Factor

$K_e = 1.0$ ----- Ground Elevation Factor

$V = 47$ m/s ----- Basic Wind Speed

$q_z = 1150$ N/m²

Design Wind Pressure, p :

$p = q_z G C_n$ ----- Design Wind Pressure, adjust C_n for windward and leeward pressure

$G = 0.8$ ----- Gust Effect Factor, assume rigid, Section 6.5.8, ASCE 7-05

$C_n = -1.8$ ----- Net Pressure Coef. determined from Fig. 6-18A, ASCE 7-05

$p = 1760$ N/m²

Snow Load, p_r :

$p_r = 0.7 C_e C_t I_s p_g$ ----- Snow Load, flat surface, refer to Ch. 7, ASCE 7-16

$C_e = 0.9$ ----- Exposure Factor

$C_t = 1.2$ ----- Thermal Factor, refer to Table 7.3-2, ASCE 7-16

$I_s = 0.8$ ----- Importance Factor, refer to Table 1.5-2, ASCE 7-16

$p_r = 289.59$ N/m²

$p_g = 478.81$ N/m² ----- Ground Snow Load, Figure 7.2-1, ASCE 7-16

$p_s = C_s p_r + p_r$ ----- Sloped Snow Load

$C_s = 0.72$ ----- Slope Factor, Figure 7.4-1, ASCE 7-16

$p_r = 0.0$ ----- Rain-on-Snow Surcharge Load

$p_s = 210.53$ N/m²

$W_c = 27.61$ m ----- Hypotenuse from Eave to Ridge

$W = 25.03$ m ----- Horizontal Distance from Eave to Ridge

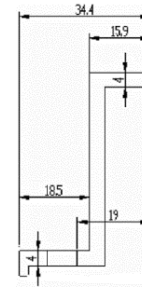
$W/50 = 1.64$ deg ----- if $p_g \leq 20$ psf, $p_g \neq 0$, tilt angle > $W/50$, $p_r = 0.0$, for rainy areas, $p_r = 5$ psf

Stress Analysis

Compression Tension Simulation (TGI Series, SAP2000)

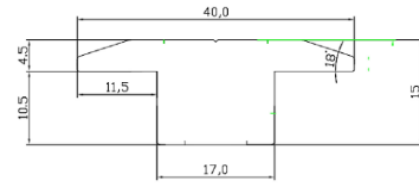
Component: End Clamp

$F_t = \frac{PA}{N_c}$ ----- Shear Force by Wind Suction
 $P = 1760 \text{ N/m}^2$ ----- Design Wind Pressure
 $A = 2.53\text{m}^2$ ----- PV Area
 $N_c = 4$ ----- Number of clamps per module
 $F_t = 1113.2 \text{ N}$
 $F_{\max} = 5972 \text{ N}$ ----- F_t does not exceed F_{\max}
 (refer to test report No. TGI-TS-002)



Component: Mid Clamp

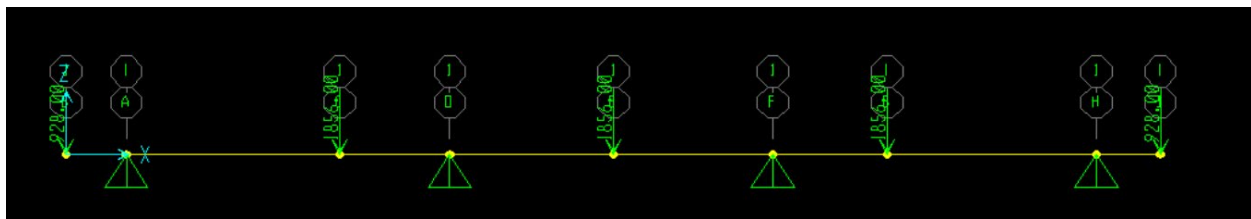
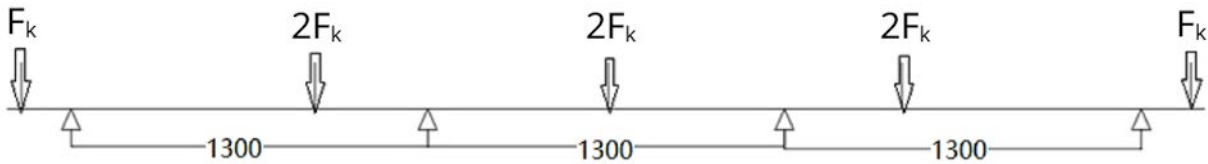
$F_t = \frac{PA}{N_c}$ ----- Shear Force by Wind Suction
 $P = 1760 \text{ N/m}^2$ ----- Design Wind Pressure
 $A = 2.53\text{m}^2$ ----- PV Area
 $N_c = 4$ ----- Number of clamps per module
 $F_t = 1113.2 \text{ N}$
 $F_{\max} = 16098 \text{ N}$ ----- F_t does not exceed F_{\max}
 (refer to test report No. TGI-TS-001)



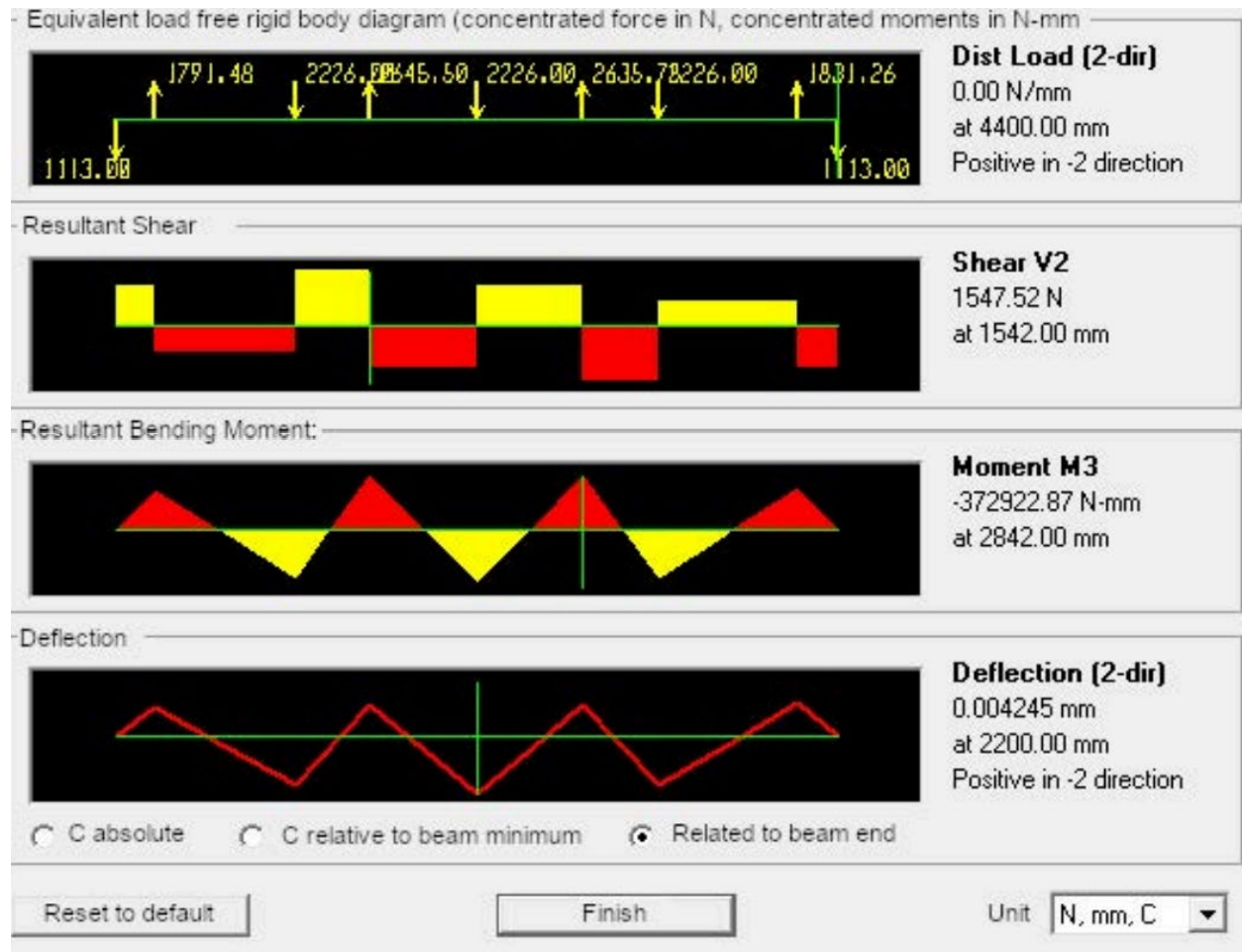
Component: Rail

6005-T5 ----- Material Property
 $\sigma = 240 \text{ MPa}$ ----- Design Strength
 $V = 205 \text{ MPa}$ ----- Design Shear
 $I_y = 88790 \text{ mm}^4$ ----- Second Moment of Inertia
 $W_y = 3288 \text{ mm}^3$ ----- Elastic Modulus
 $A = 292 \text{ mm}^2$ ----- Sectional Area
 $L_d = 1300 \text{ mm}$ ----- Span

Free Body Diagram:



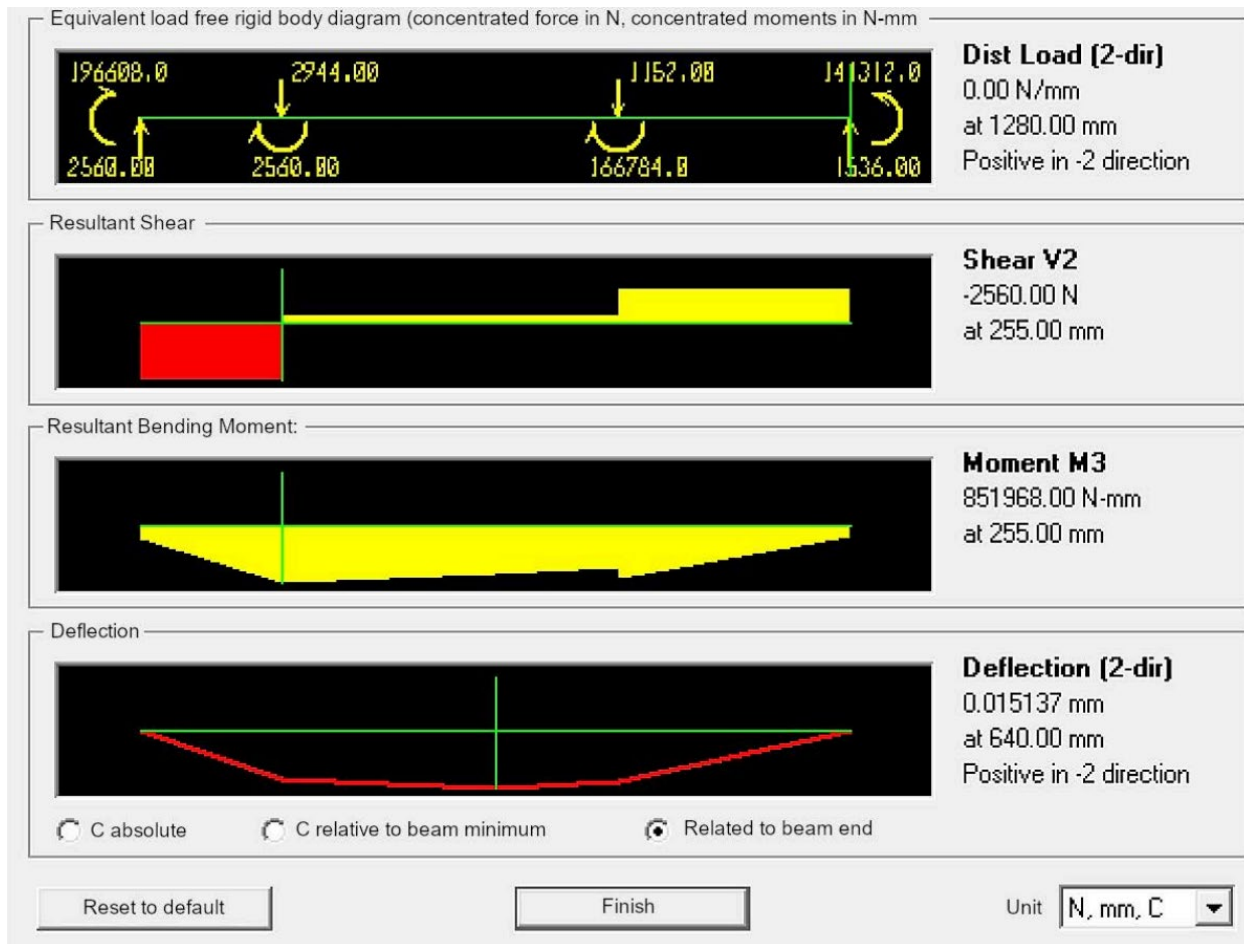
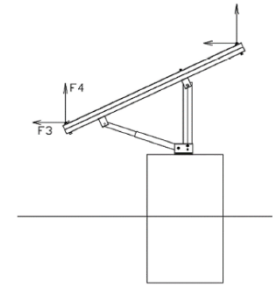
$F_k = \frac{PA}{N_c}$ ----- Shear Force by Wind Suction
 $F_k = 1113 \text{ N}$



$M_{max} = 310936 \text{ Nm}$ ----- Max Moment
 $\sigma_{max} = \frac{M_{max}}{W_y}$ ----- Max Stress
 $\sigma_{max} = 240 \text{ MPa}$ ----- Does not exceed max load capacity

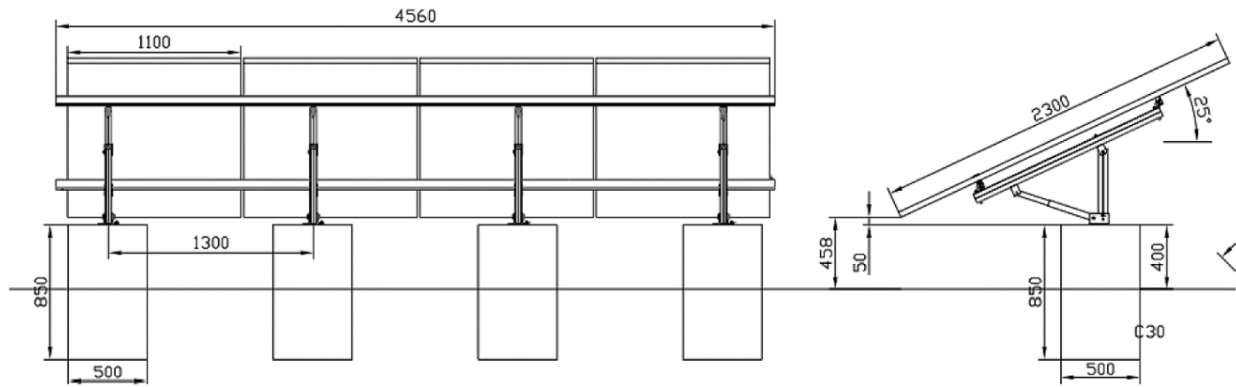
Component: Main Beam

$F_3 = \frac{PA^4 \sin \theta}{8}$ ----- Force in x-direction
 $P = 1760 \text{ N/m}^2$ ----- Design Wind Pressure
 $A = 2.53\text{m}^2$ ----- PV Area
 $\theta = 45 \text{ deg}$ ----- Angle w.r.t. PV
 $F_3 = 1574 \text{ N}$
 $W_{y1} = 7505 \text{ m}^3$ ----- Elastic Modulus



$M_{max} = 851968 \text{ Nm}$ ----- Max Moment
 $\sigma_{max} = \frac{M_{max}}{W_{y1}}$ ----- Max Stress
 $\sigma_{max} = 240 \text{ MPa}$ ----- Does not exceed max load capacity

Component: Cement Base



$$F_5 = \frac{PA^4 \cos \theta}{8} \text{ ----- Force in x-direction}$$

$P = 1760 \text{ N/m}^2$ ----- Design Wind Pressure
 $A = 2.53\text{m}^2$ ----- PV Area
 $\theta = 25 \text{ deg}$ ----- Angle w.r.t. PV
 $F_5 = 16137 \text{ N}$
 $W_p = 300 \text{ N}$ ----- Weight of PV
 $W_c = 5100 \text{ N}$ ----- Weight of cement base
 $F_5 < 4W_p + 4W_c$
 $F_5 < 21600 \text{ N}$ ----- Does not exceed max load capacity

Report No. TGI-TS-002

TEST REPORT

Received No.: 62-01-083
Report No: TGI-TS-002
Rev.:00

Customer's company name : Sunforson Power CO.,LTD
Address : 20/27 Soi Bang Na-Trat 56 Bangna District T.Bangkaew A.Bangplee Bangkok,
Thailand

Part Name:	End Clomp	Material Characterization:	-
Part No:	-	Standard of Test:	-
Supplier:	-	Type of Test:	Tensile Testing
Material Specification:	-	Equipment / Serial No.:	Universal Testing Machine
Received Date:	24 / Jan. / 2019	Equipment's Capacity:	Tension force 10 tons
Tested Date:	07 / Feb. / 2019	Ambient Temp.:	23.5 °C,65 % RH



Result of Tensile Test

Test No.	Max Load (N)	Max Load (kgf)
1	6,150.779	626.990
2	5,971.995	608.766
3	6,178.216	629.787

Operated by: Sunapit
Testing Operator
Technical Team

Checked by: Sut Chon
Mr.Arun Jeangsrijaoen
Technical Manager

Approved by: Sut Chon
Mr.Arun Jeangsrijaoen
Quality Manager

Report No. TGI-TS-001

TEST REPORT

Received No.: 62-05-113

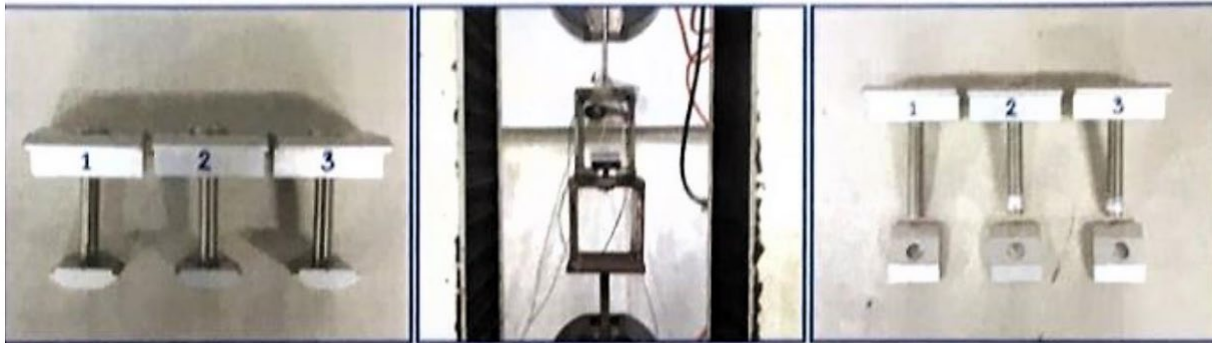
Report No: TGI-TS-001

Rev.:00

Customer's company name : **Sunforson Power CO.,LTD**

Address : **20/27 Soi Bang Na-Trat 56 Bangna District T.Bangkaew A.Bangplee Bangkok, Thailand**

Part Name:	Mid Clamp	Material Characterization:	-
Part No:	-	Standard of Test:	-
Supplier:	-	Type of Test:	Tensile Testing
Material Specification:	-	Equipment / Serial No.:	Universal Testing Machine
Received Date:	30 / May / 2019	Equipment's Capacity:	Tension force 10 tons
Tested Date:	06 / June / 2019	Ambient Temp.:	24.7 °C,62 % RH



Result of Tensile Test

Test No.	Max Load (N)	Max Load (kgf)
1	16,098.056	1,640.984
2	20,706.604	2,110.764
3	20,379.571	2,077.428